TABLE 1: TABLE OF REINFORCING STEEL DIMENSIONS AND DATA																
DESIGN H	6′	8′	10'	12′	14′	16′	18′	20′	22'	24′	26′	28′	30′	32′	NOTE:	
W	8'- 3"	8'- 6"	9'- 0"	9'- 6"	10'- 0"	10'- 9"	11'- 3"	12'- 0"	13'- 3"	14'- 3"	15'- 9"	16'- 9"	18'- 0"	19'- 9"		e ehown
С	2'- 9"	2'- 9"	3'- 0"	3'- 3"	3'- 4"	3'- 6"	3'- 9"	4'- 0"	4'- 3"	4'- 9"	5'- 3"	5'- 6"	5'- 9"	6'- 7"	Total @ bars and f bars are total number of top	p and
В	5'- 6"	5'- 9"	6'- 0"	6'- 3"	6'- 8"	7'- 3"	7′- 6''	8'- 0"	9'- 0"	9'- 6"	10'- 6"	11'- 3"	12'- 3"	13'- 2"	bottom bars combined.	
F PILE FOOTING	1'- 6"	1'- 6"	1'- 6"	1'- 6"	1'- 9"	2'- 0"	2'- 0"	2'- 6"	2'- 9"	2'- 9"	3'- 0"	3'- 3"	3'- 9"	4'- 0"	LEGEND:	
M	1'- 3"	1'- 6''	1'- 6"	1'- 9"	1'- 10"	2'- 0''	2'- 3"	2'- 6"	2'- 9"	3'- 3"	3'- 9"	4'- 0''	4'- 3"	5'- 1"	8:2 bar bundle	
N	4'- 0"	4'- 3''	4'- 6"	4'- 9"	5'- 2"	5'- 9"	6'- 0"	6'- 6"	7′- 6''	8'-0"	9'-0"	9'- 9"	10'- 9"	11'- 8"	0 · 2 bar barrare	
ROW 1 SPACING	12'- 3"	10'- 3"	8'- 9"	7′- 6''	6'- 3"	5'- 3"	4'- 9"	4'- 0"	3'- 9"	3'- 9"	4'-0"	3'- 9"	3'- 9"	3'- 9"		
ROW 2 SPACING	14'- 0"	12'- 9"	11'- 6"	10'- 3"	9'- 3"	8'- 3"	7′- 9''	6'- 6"	7′- 6''	6'- 0"	4'- 0"	4'- 0''	3'- 9"	3'-9"		
ROW 3 SPACING									6'- 0"	5′- 3''	5′-0''	4'- 0"	6'- 0"	4'- 0"		
ROW 4 SPACING													3'- 9"	3'-9"		
STEM WITH HAUNCH, BATTER	0	1/2:12	1/2 : 12	1/2:12	1/2:12	½: 12	√2 : 12	1/2 : 12	√2 : 12	5⁄% : 12	<i>3</i> / ₄ :12	⅓: 12	1:12	1:12		
STEM WITHOUT HAUNCH, BATTER	0	0	0	0	0	0	0	0	1/4:12	1/4:12	1/2 : 12	3/4:12	3/4:12	3/4:12		
BARS						#7 @ 15	#7 @ 12	#7 @ 12	#8 @ 12	#6 @ 6	#6 @ 6	#6 @ 6	#8 @ 9	#9 @ 9	DE (SIGN
(b) BARS	#8 @ 12	#7 @ 9	#7 @ 6	#7 @ 6	#7 @ 6	#9 @ 7.5	#9 @ 6	#10 @ 6	#10 @ 6	#8 @ 6 B	#8 @ 6 8	#8 @ 65	#10 @ 9{	#11 @ 9 8	<u> </u>	31011
ha			5'- 0"	6'- 0"	7'- 0"	7' - 0"	6'- 0''	7'- 0"	7'- 0"	7'- 6"	8'- 6"	9'- 3"	15'- 0"	11'- 3"	Dosi	gn: AASH1
hb						11' - 6"	12'- 0"	13'- 3"	16'- 0"	15'- 6"	17'- 6''	18'- 9"	21'- 0"	20'- 9"	Desi	4th 6
© BARS	#6 @ 12	#6 @ 9	#6 @ 6	#6 @ 6	#6 @ 6	#8 @ 7.5	#8 @ 6	#9 @ 6	#9 @ 6	#10 @ 6	#10 @ 6	#11 @ 6	#10 @ 9 \	#10 @ 9 K	ws:	33 ps
@ BARS	#5 @ 12	#5 @ 9	#5 @ 12	#5 @ 12	#5 @ 12	#6 @ 15	#5 @ 12	#5 @ 12	#6 @ 12	#6 @ 12	#6 @ 12	#7 @ 12	#6 @ 9	#9 @ 9		•
BARS	10-#7 @ 6	8-#7 @ 7	10-#6 @ 6	8-#6 @ 6	6-#6 @ 12	2 6-#5 @ 12	6-#5 @ 12	6-#5 @ 15	#5 @ 18	#5 @ 18	#5 @ 18	#5 @ 18	#5 @ 18	#5 @ 18	LS:	Varie
① BARS	10-#8 @ 7	10-#8 @ 6	10-#7 @ 8	12-#6 @ 7	8-#7 @ 1	17-#6 @ 13	8-#6 @ 12	8-#5 @ 15	#5 @ 18	#5 @ 18	#5 @ 18	#5 @ 18	#5 @ 18	#5 @ 18	CT:	54 ki distr

DESIGN DATA

Design: AASHTO LRFD Bridge Design Specifications 4th edition with California Amendments

33 psf on Sound Wall and Barrier

COUNTY

PLANS APPROVAL DATE

LS: Varied surcharge on level ground surface

54 kip maximum traffic impact loading evenly distributed over 10 feet at top of the barrier and 1:1 distribution down and outward

REGISTERED CIVIL ENGINEER DATE

The State of California or its officers or agents

mpleteness of electronic copies of this plan shee:

shall not be responsible for the accuracy or

CIVIL

Mononabe-Okabe Method

 K_h = 0.3 K_V = 0.0

 $\emptyset = 34^{\circ}$ Soil: $\gamma = 120 \text{ pcf}$

Reinforced Concrete: f'c = 3600 psi

fy = 60,000 psi

Load Combinations and Limit States

Q=1.00DC+1.00EV+1.00EH+1.00LS+0.30WS Service I

Q=1.00DC+1.00EV+1.00EH+1.00WS Service II

Strength I

Q=aDC+BEV+1.50EH+1.75LS Q=1.25DC + 1.35EV + 0.90EH +1.75LS

(for piles at heel)

Strength III Q=aDC+BEV+1.50EH+1.40WS

Strength V Q=aDC+BEV+1.50EH+1.35LS+0.40WS

Q=1.00DC+1.00EV+1.00EH+1.00EQD+1.00EQE Extreme I

Q=1.00DC+1.00EV+1.00EH+1.00CT Extreme II

Where:

CONTRACT NO.: X

Force Effects 0:

1.25 or 0.90, Which ever Controls Design 1.35 or 1.00, which ever Controls Design Dead Load of Structure Components a:

EV: Vertical Earth Fill Pressure

LS: Live Load Surcharge

EQE: Seismic Earth Pressure

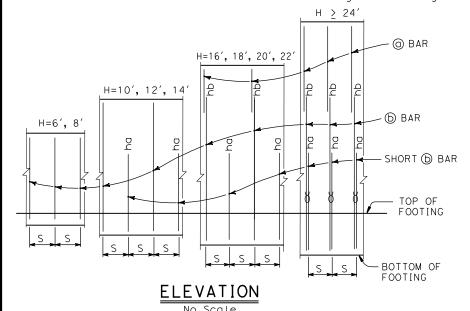
EQD: Soil and Structure Components Inertia. Soil inertia ignored for stem design

Wind Load on Sound Wall and Barrier

CT: Vehicular Collision Force

NOTES:

- 1. All piles are class 90 concrete piles.
- 2. Pile batter shown are 1:3.
- 3. Minimum distance between center pile and edge of footing is 1'-6".
- 4. Lateral resistance of each pile: 30 kip for strength limit states. 40 kip for extreme limit states.
- 5. Maximum spacing between piles is shown in the table. Reduce to suit the length of footing.
- 6. Minimum distance between any two piles is 3'-0". Reduce to suit the length of footing.



"ha" and "hb" above (b) bars indicate distance from top of footing to upper end of (b) bars, see table. "S" is (b) bar spacing, see table. (c): 2 bar bundle

7. For sound wall and retaining wall architectural finish or texture, see details elsewhere in Project Plans.

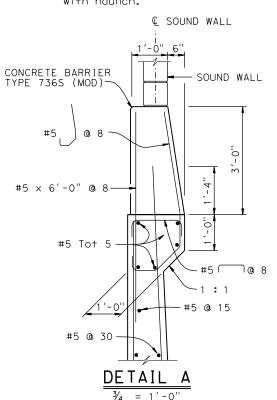
8. For details not shown and drainage notes, see / B3-5

9. Footing cover, 1'-6" minimum.

10. For sound wall and Barrier reinforcements see "SOUND WALL -MASONRY BLOCK WITH BARRIER ON RETAINING WALL" sheets.

11. For H=6' through 14', extend (1) bars into Barrier for stem with haunch.

12. For H≥16', extend @ bars into Barrier for stem with haunch.



SOUND WALL CONCRETE BARRIER TYPE 736S (MOD) 15 @ 8 #5 x 6'-0" @ 8 T @ 8 #5 Cont, TOT 8 -BATTER BACKFACE #5 @ 15 #5 @ 30 OPTIONAL DETAIL A

For Details not shown, see "DETAIL A"

ROJECT NUMBER & PHASE: X

FILE => \$REQUEST

STANDARD DRAWING BRIDGE NO. STATE OF X DIVISION OF CALIFORNIA FILE xs14-320-1 **ENGINEERING SERVICES** RETAINING WALL TYPE 1SWBP-DETAILS NO. 1 DEPARTMENT OF TRANSPORTATION PPROVAL DATE July 2011 DS OSD 2147A (ENGLISH STANDARD DRAWING "XS" BORDER REV. (02-02-11) ORIGINAL SCALE IN INCHES DISREGARD PRINTS BEARING EARLIER REVISION DATES